

# Dynamic analysis of viscoelastic concrete plates containing nanoparticle subjected to low velocity impact load

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**Abstract.** Dynamic study of concrete plates under impact load is presented in this article. The main objective of this work is presenting a mathematical model for the concrete plates under the impact load. The concrete plate is reinforced by carbon nanoparticles which the effective material properties are obtained by mixture's rule. Impacts are assumed to occur normally over the top layer of the plate and the interaction between the impactor and the structure is simulated using a new equivalent three-degree-of-freedom (TDOF) spring-mass-damper (SMD) model. The structure is assumed viscoelastic based on Kelvin-Voigt model. Based on the classical plate theory (CPT), energy method and Hamilton's principle, the motion equations are derived. Applying DQM, the dynamic deflection and contact force of the structure are calculated numerically so that the effects of mass, velocity and height of the impactor, volume percent of nanoparticles, structural damping and geometrical parameters of structure are shown on the dynamic deflection and contact force. Results show that considering structural damping leads to lower dynamic deflection and contact force. In addition, increasing the volume percent of nanoparticles yields to decreases in the deflection.

**Keywords:** concrete plate; CPT; low velocity impact; nanoparticles; numerical method

## 1. Introduction

One of the most common structural elements that may be exposed to impact is plates. Plates are widely used in building structures and bridges and are usually made of concrete. Impact load is a kind of impulsive dynamic load which is disregarded in the design process of plates like other structural members. The main reason for the lack of sufficient research to explore the effect of impact on plates is the fact that the analyses and design of structures subjected to dynamic impact loading are generally very complicated, and such investigations are more difficult when working with inelastic materials, such as concrete.

The dynamic behavior of axially loaded concrete reinforced square column was studied by Amoli *et al.* (2018) experimentally. The deformation mechanics of the composite sandwich beams with smart composite face sheets was calculated by Arbabi *et al.* (2017). An experimental work for considering the dynamic behaviour of reinforced aggregate concrete beams was presented by Azmi *et al.* (2019). Bakhshande Amnieh *et al.* (2018) studied the dynamic response of reinforced concrete plate under blast. A novel three-dimensional (3D) method to simulate projectile penetration into the reinforced concrete

beam was considered by Faramoushjan *et al.* (2021). Numerical studies on the vibration of plate was prepared by Farokhian and Kolahchi (2020). The behavior of different reinforced and unreinforced smart plates was reviewed by Fakhar and Kolahchi (2018) based on experiments by Split Hopkinson Pressure Bar. Golabchi *et al.* (2018) proposed an efficient FE model of pre-stressed concrete plate under missile impact considering the effects of damping, erosion, and pre-stressing values (Ghorbanpour Arani *et al.* 2016). Hajmohammad *et al.* (2017, 2018a, b, c, 2019a, b, 2021) focused on the effects of the support types and the support layouts on the dynamic responses and the failure modes of plates subjected to dynamical loads. Heidarzadeh *et al.* (2018) investigated the influence of various parameters such as the rebar's material, amount and arrangement of GFRP, concrete strength on mechanical behavior of reinforced concrete plates. Jafarian Arani (2016) investigated the combination effect of steel and FRP bars on impact response of concrete plates. Numerical results revealed that using hybrid FRP-steel reinforcing grows energy absorption of plate. Jassas *et al.* (2019) executed both experimental and numerical study for considering the impact behavior of two ways simply supported CFRP strips strengthened RC plates. Jamali *et al.* (2016, 2019) developed a failure analytical model to assess the characteristics of reinforced concrete plates under oblique perforation by a rigid projectile. Javano *et al.* (2019) presented the impact response of CFRP-strengthened RC panels under the impact of non-deformable projectiles.

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Based on an analytical approach, the free Low velocity impact and transient response of FG piezoelectric cylindrical panels subjected to impulsive loads were presented by Jafari Natanzi (2018). Keshtegar *et al.* (2018, 2020a, b, c) investigated active Low velocity impact control of piezoelectric bonded smart structures using PID algorithm. Kolahdouzan *et al.* (2020) considered the impact force on the CNT-reinforced composite plates integrated with piezoelectric layers using Reddy's higher-order shear deformation theory. Motezaker *et al.* (2017a, b, 2021) carried out the analysis of dynamic responses of CNT reinforced composite plates subjected to impact loading. Naseri Taheri *et al.* (2020) studied experimental study on shear behaviors of partial precast steel reinforced concrete beams. Taherifar *et al.* (2020, 2021) presented structural lightweight concrete containing expanded poly-styrene beads.

None of the above mentioned works have studied the impact response of the concrete plates based on numerical solution. The aims and objectives of this paper are presenting a mathematical model for the concrete plates under impact load, numerical solution based on DQM, assuming the effect of nanoparticles and assuming a comprehensive model for the impact simulation. In the present study, low velocity impact of concrete plate is investigated. The concrete plate is reinforced by carbon nanoparticles which the effective material proprieties are obtained by mixture's rule. Applying CPT, the motion equations are obtained based on Hamilton's principal. Spring – mass models are used extensively for analyzing the dynamics of impact. DQM is applied for obtaining the dynamic deflection and contact force of the system. The effects of mass, velocity and height of impactor, boundary conditions, structural damping and geometrical parameters of structure are shown on the dynamic deflection and contact force of system.

## 2. Strain relations

The schematic of concrete plate containing nanoparticle with length  $a$ , width  $b$  and thickness  $h$  is shown in Fig. 1 subjected to an impactor with mass of  $m$ , velocity of  $V_0$  and height of  $H$ .

Based on CPT, the displacement field can be expressed as (Mosharrafian *et al.* 2016, Zamanian *et al.* 2017, Zamani *et al.* 2017, Zarei *et al.* 2017)

$$U_1(x, y, z, t) = U(x, y, t) - z \frac{\partial W}{\partial x} \quad (1)$$

$$U_2(x, y, z, t) = V(x, y, t) - z \frac{\partial W}{\partial y} \quad (2)$$

$$U_3(x, y, z, t) = W(x, y, t), \quad (3)$$

where  $(U, V)$  denote the displacement components in  $x$ - and  $y$ - directions, respectively. In addition,  $W$  are bending and shear transverse deflections, respectively. Based on above relations, the strain-displacement equations may be written as

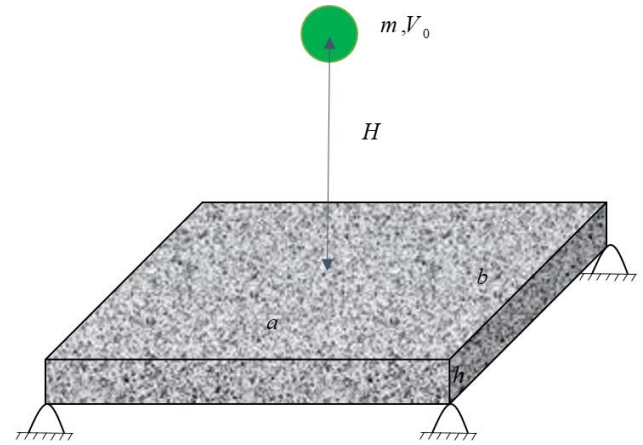


Fig. 1 A schematic of concrete plate subjected to an impactor containing nanoparticle

$$\begin{pmatrix} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \gamma_{xy} \end{pmatrix} = \begin{pmatrix} \varepsilon_{xx}^0 \\ \varepsilon_{yy}^0 \\ \gamma_{xy}^0 \end{pmatrix} + z \begin{pmatrix} k_{xx} \\ k_{yy} \\ k_{xy} \end{pmatrix} \quad (4)$$

$$\begin{pmatrix} \gamma_{yz} \\ \gamma_{xz} \end{pmatrix} = \begin{pmatrix} 0 \\ 0 \end{pmatrix} \quad (5)$$

where  $\varepsilon_{ij}^0$ ,  $k_{ij}^b$  and  $k_{ij}^s$  are mid plane strains, bending and shear curvatures, respectively which are

$$\begin{pmatrix} \varepsilon_{xx}^0 \\ \varepsilon_{yy}^0 \\ \gamma_{xy}^0 \end{pmatrix} = \begin{pmatrix} \frac{\partial U}{\partial x} \\ \frac{\partial V}{\partial y} \\ \frac{\partial U}{\partial y} + \frac{\partial V}{\partial x} \end{pmatrix}, \quad \begin{pmatrix} k_{xx} \\ k_{yy} \\ k_{xy} \end{pmatrix} = \begin{pmatrix} -\frac{\partial^2 W}{\partial x^2} \\ -\frac{\partial^2 W}{\partial y^2} \\ -2\frac{\partial^2 W}{\partial x \partial y} \end{pmatrix}, \quad (6a)$$

$$\begin{pmatrix} \gamma_{yz}^s \\ \gamma_{xz}^s \end{pmatrix} = \frac{-4}{h^3} \begin{pmatrix} \frac{\partial W_s}{\partial y} \\ \frac{\partial W_s}{\partial x} \end{pmatrix} \quad (6b)$$

## 3. Stress-strain relations

Based on Hook's law, the stress-strain relation of the concrete plate can be written as (Kolahchi *et al.* 2020):

$$\begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{zz} \\ \sigma_{zy} \\ \sigma_{xz} \\ \sigma_{xy} \end{bmatrix} = \begin{bmatrix} C_{11} & C_{12} & C_{13} & 0 & 0 & 0 \\ C_{12} & C_{22} & C_{23} & 0 & 0 & 0 \\ C_{13} & C_{23} & C_{33} & 0 & 0 & 0 \\ 0 & 0 & 0 & C_{44} & 0 & 0 \\ 0 & 0 & 0 & 0 & C_{55} & 0 \\ 0 & 0 & 0 & 0 & 0 & C_{66} \end{bmatrix} \begin{bmatrix} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \varepsilon_{zz} \\ \gamma_{zy} \\ \gamma_{xz} \\ \gamma_{xy} \end{bmatrix}, \quad (7a)$$

where  $\sigma_{ij}$ ,  $\varepsilon_{ij}$  and  $C_{ij}$  are stress, strain and elastic

constants, respectively. The elastic constants can be obtained by Mixture's rule as:

$$C_{ij} = C_{ij}^n V_n + C_{ij}^m (1 - V_n) \quad (7b)$$

where super index "n": is related to nanoparticles and super index "m" is related to matrix. In addition,  $V_n$ , is volume percent of nanoparticle.

#### 4. Energy method

The potential energy ( $U$ ) can be written as

$$U = \frac{1}{2} \int_A \int_{-\frac{h}{2}}^{\frac{h}{2}} \left( \sigma_{xx} \varepsilon_{xx} + \sigma_{yy} \varepsilon_{yy} + \sigma_{xy} \gamma_{xy} + \sigma_{xz} \gamma_{xz} + \sigma_{yz} \gamma_{yz} \right) dz dA \quad (8)$$

Combining of Eqs. (1)-(3) and (8) yields

$$U = \frac{1}{2} \int_A \left( N_{xx} \frac{\partial U}{\partial x} + N_{yy} \frac{\partial U}{\partial y} + N_{xy} \frac{\partial V}{\partial x} + N_{xy} \frac{\partial V}{\partial y} - M_{xx} \frac{\partial^2 W}{\partial x^2} - M_{yy} \frac{\partial^2 W}{\partial y^2} - 2M_{xy} \frac{\partial^2 W}{\partial y \partial x} \right) dA, \quad (9)$$

where the stress resultant-displacement relations can be written as

$$\begin{bmatrix} N_{xx} \\ N_{yy} \\ N_{xy} \end{bmatrix} = \int_{-h}^h \begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{bmatrix} dz \quad (10)$$

$$\begin{bmatrix} M_{xx} \\ M_{yy} \\ M_{xy} \end{bmatrix} = \int_{-h}^h \begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{bmatrix} z dz \quad (11)$$

Substituting stress-strain relation into Eqs. (10) and (11), the stress resultant-displacement relations can be obtained as follow

$$N_{xx} = A_{11} \frac{\partial}{\partial x} U - A_{11z} \frac{\partial^2}{\partial x^2} W + A_{12} \frac{\partial}{\partial y} V - A_{12z} \frac{\partial^2}{\partial y^2} W, \quad (12)$$

$$N_{yy} = A_{21} \frac{\partial}{\partial x} U - A_{21z} \frac{\partial^2}{\partial x^2} W + A_{22} \frac{\partial}{\partial y} V - A_{22z} \frac{\partial^2}{\partial y^2} W, \quad (13)$$

$$N_{xy} = A_{44} \frac{\partial}{\partial y} U + A_{44} \frac{\partial}{\partial x} V - 2A_{44z} \frac{\partial^2}{\partial x \partial y} W \quad (14)$$

$$M_{xx} = A_{11z} \frac{\partial}{\partial x} U - B_{11} \frac{\partial^2}{\partial x^2} W - A_{12z} \frac{\partial}{\partial y} V - B_{12} \frac{\partial^2}{\partial y^2} W, \quad (15)$$

$$M_{yy} = A_{21z} \frac{\partial}{\partial x} U - B_{21} \frac{\partial^2}{\partial x^2} W + A_{22z} \frac{\partial}{\partial y} V - B_{22} \frac{\partial^2}{\partial y^2} W \quad (16)$$

$$M_{xy} = 2A_{44z} \frac{\partial}{\partial y} U + 2A_{44z} \frac{\partial}{\partial x} V - 2B_{44} \frac{\partial^2}{\partial x \partial y} W, \quad (17)$$

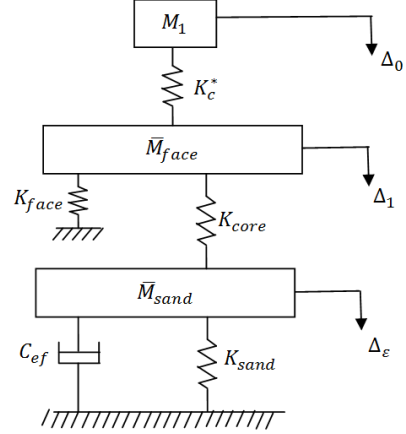


Fig. 2 The equivalent TDOF model of the structure and the impactor system (SMD model)

where

$$(A_{11}, A_{12}, A_{22}, A_{44}) = \int_{-h}^h (C_{11}, C_{12}, C_{22}, C_{44}) dz, \quad (18)$$

$$(A_{11z}, A_{12z}, A_{22z}, A_{44z}) = \int_{-h}^h (C_{11}, C_{12}, C_{22}, C_{44}) z dz, \quad (19)$$

$$(B_{11}, B_{12}, B_{22}, B_{44}) = \int_{-h}^h (C_{11}, C_{12}, C_{22}, C_{44}) z^2 dz, \quad (20)$$

The kinetic energy ( $K$ ) of system may be written as

$$K = \frac{1}{2} \rho \int_A \int_{-\frac{h}{2}}^{\frac{h}{2}} \left( \left( \frac{\partial u}{\partial t} \right)^2 + \left( \frac{\partial v}{\partial t} \right)^2 + \left( \frac{\partial w}{\partial t} \right)^2 \right) dz dA. \quad (21)$$

#### 5. Low-velocity impact of structure

As shown in Fig. 2, the three-degrees-of-freedom (TDOF) spring-mass-damper (SMD) model is applied to predict the low-velocity impact response of concrete plate (Xu *et al.* 2020).

The motion equations of the three-TDOF system can be written as follows:

$$M_1 \ddot{\Delta}_0 + K_c^* (\Delta_0 - \Delta_1) = 0 \quad (22)$$

$$\begin{aligned} \bar{M}_{face} \ddot{\Delta}_1 + K_{face} \Delta_1 + K_{face} (\Delta_1 - \Delta_2) \\ + K_c^* (\Delta_1 - \Delta_0) = 0 \end{aligned} \quad (23)$$

$$\bar{M}_{sand} \ddot{\Delta}_2 + K_{sand} \Delta_2 + K_{core} (\Delta_2 - \Delta_1) + C_{ef} \dot{\Delta}_2 = 0 \quad (24)$$

where  $\Delta_0$ ,  $\Delta_1$  and  $\Delta_2$  are transverse displacements of impactor, impacted top and bottom face sheets, respectively,  $M_1$ ,  $\bar{M}_{face}$  and  $\bar{M}_{sand}$  are the mass of impactor, effective mass of impacted face sheet and effective mass of concrete plate, respectively,  $K_c^*$ ,  $K_{face}$ ,  $K_{sand}$  and  $K_{core}$  are respectively, effective stiffness of contact, face sheet, concrete plate and core,  $C_{ef}$  is the effective viscous damping coefficient which can be defined as

$$C_{ef} = \eta_{st} (K_{stand} / \omega_1) \quad (25)$$

where  $\eta_{st}$  and  $\omega_1$  are damping coefficient and fundamental natural frequency of structure, respectively. The effective stiffness of impactor can be expressed as

$$K_C^* = \sqrt{\pi} \Gamma \left( \frac{P+1}{2} \right) \frac{2\Gamma \left( \frac{P}{2} + 1 \right) + \sqrt{\pi} \Gamma \left( \frac{P+1}{2} \right)}{4\Gamma^2 \left( \frac{P}{2} + 1 \right) + \pi \Gamma^2 \left( \frac{P+1}{2} \right)} \alpha_{max}^{P-1} K_C \quad (26)$$

where  $\Gamma$  is gamma function,  $P$  is Hertzian indentation (usually  $P=1.5$ ),  $\alpha_{max}$  is a parameter which can be written as:

$$\alpha_{max} = \left( \frac{\bar{M}_{sand} \cdot M_1}{\bar{M}_{sand} + M_1} \right)^{\frac{1}{(P+1)}} \left( \frac{P+1}{2} \right)^{\frac{1}{(P+1)}} \left( \frac{V_0^2}{K_C} \right)^{\frac{1}{(P+1)}} \quad (27)$$

where  $V_0$  is the impact velocity of impactor and the contact stiffness ( $K_C$ ) may be estimated as

$$K_C = \left( \frac{4}{3} \right) E_I R_I^{1/2}, \frac{1}{E_I} = \frac{1 - \nu_I^2}{E_I} + \frac{1 - \nu_P^2}{E_P} \quad (28)$$

where  $R_I, E_I$  and  $\nu_I$  are radius, elastic modulus and poisson's ratio of impactor, respectively,  $E_P$  and  $\nu_P$  are elastic modulus and poisson's ratio of structure, respectively.

The effective compressive stiffness of elastic flexible core can be given as follows (Chow *et al.* 1992):

$$K_{core} = 8 \sqrt{k_F D_f^*} \quad (29)$$

where  $k_F$  is the foundation stiffness (elastic region of the core is modeled as a Winkler foundation) and  $D_f^*$  is the effective stiffness of the impacted face sheet which can be written as

$$D_f^* = \sqrt{D_{11} D_{22} (\gamma + 1) / 2} \quad (30)$$

$$\gamma = (D_{12} + 2D_{66}) / \sqrt{D_{11} D_{22}} \quad (31)$$

$$k_F = \frac{E_c}{\bar{h}_c} \quad (32)$$

where  $D_{ij}$  are bending rigidity constants,  $E_c$  is the concrete Young's modulus and  $\bar{h}_c$  can be expressed as

$$\bar{h}_c = \frac{h_c}{1.38} \quad \text{for } h_c \leq h_{c \max} \quad (33)$$

and  $\bar{h}_c = 2h_{c \max}$  for  $h_c > h_{c \max}$

$$\bar{h}_c = \left( \frac{27}{64} \right)^2 2h_{c \max} \quad \text{for } h_c > h_{c \max} \quad (34)$$

$$h_{c \max} \approx h_f \left( \frac{32}{27} \right) \left( \frac{4Q_f^*}{3E_c} \right)^{1/3} \quad \text{where } Q_f^* = \frac{12D_f^*}{h_f^3} \quad (35)$$

where  $h_f$  is the thickness of the impacted face sheet. The system of ordinary differential equation can be solved analytically using the following initial conditions:

$$\Delta_0(t=0) = 0, \Delta_1(t=0) = 0, \Delta_2(t=0) = 0, \quad (36)$$

$$\dot{\Delta}_0(t=0) = V_0, \dot{\Delta}_1(t=0) = 0, \dot{\Delta}_2(t=0) = 0, \quad (37)$$

By applying the equivalent, the eigenvalue equation can be obtained. Therefore,

$$(M_I \bar{M}_{face} \bar{M}_{sand}) \lambda^{*3} - \left[ \begin{array}{c} K_{gbc} \bar{M}_{face} M_I + K_{gcc} \bar{M}_{sand} M_I \\ + K_C^* \bar{M}_{face} \bar{M}_{sand} \end{array} \right] \lambda^{*2} + \left[ \begin{array}{c} M_I (K_{gbc} K_{gbc} - K_{core}^2) \\ + K_C^* (K_{gbc} \bar{M}_{face} + K_{gcc} \bar{M}_{sand}) - K_C^{*2} \bar{M}_{sand} \end{array} \right] \lambda^* + [K_C^{*2} K_{gbc} - K_C^* (K_{gcc} K_{gbc} - K_{core}^2)] = 0 \quad (38)$$

where

$$K_{gcc} = K_{face} + K_{core} + K_C^* \quad (39)$$

$$K_{gbc} = K_{sand} (1 + \eta_{stj}) + K_{core} \quad (40)$$

$$j = \sqrt{-1} \quad (41)$$

The above eigenvalue equation has complex coefficients of  $\lambda^* = \lambda' + i\lambda''$  where the circular frequency is  $\omega = \sqrt{\lambda'}$ . Finally, the equivalent contact force can be obtained as

$$[F_c^*(t) = K_C^* [c_1 (\phi_0^1 - 1) \sin(\omega_1 t) + c_2 (\phi_0^2 - 1) \sin(\omega_2 t) + c_3 (\phi_0^3 - 1) \sin(\omega_3 t)]] \quad (42)$$

where

$$\phi_0^i = \text{real} \left( \frac{K_C^*}{K_C^* - M_I \lambda^*} \right), \quad (43)$$

$$\phi_0^i = \text{real} \left( \frac{K_{core}}{K_{gbc} - \bar{M}_{sand} \lambda^*} \right), \quad i = 1, 2, 3$$

$$c_1 = \frac{-V_0 (\phi_2^2 - \phi_2^3)}{\omega_1 [(\phi_0^2 - \phi_0^1) (\phi_2^1 - \phi_2^3) - (\phi_2^2 - \phi_2^1) (\phi_0^1 - \phi_0^3)]} \quad (44)$$

$$c_2 = \frac{V_0 (\phi_2^1 - \phi_2^3)}{\omega_2 [(\phi_0^2 - \phi_0^1) (\phi_2^1 - \phi_2^3) - (\phi_2^2 - \phi_2^1) (\phi_0^1 - \phi_0^3)]} \quad (45)$$

$$c_3 = \frac{-V_0 (\phi_2^1 - \phi_2^2)}{\omega_3 [(\phi_0^2 - \phi_0^1) (\phi_2^1 - \phi_2^3) - (\phi_2^2 - \phi_2^1) (\phi_0^1 - \phi_0^3)]} \quad (46)$$

The governing equations can be derived by Hamilton's principal as follows

$$\int_0^t (\delta U - \delta K - \delta W_e) dt = 0. \quad (47)$$

where  $W_e$  is the external force by impact load. Substituting above relations into Eq. (47) yields the following governing equations

$$\frac{\partial}{\partial x} N_{xx} + \frac{\partial}{\partial y} N_{xy} - I_0 \frac{\partial^2 U}{\partial t^2} + I_1 \frac{\partial^3 W}{\partial x \partial t^2} = 0, \quad (48)$$

$$\frac{\partial}{\partial x} N_{xy} + \frac{\partial}{\partial y} N_{yy} - I_0 \frac{\partial^2 V}{\partial t^2} + I_1 \frac{\partial^3 W}{\partial y \partial t^2} = 0, \quad (49)$$

$$\frac{\partial^2}{\partial x^2} M_{xx} + 2 \frac{\partial^2}{\partial x \partial y} M_{xy} + \frac{\partial^2}{\partial y^2} M_{yy} - I_0 \left( \frac{\partial^3 W}{\partial t^2} \right) - I_1 \left( \frac{\partial^3 U}{\partial x \partial t^2} + \frac{\partial^3 V}{\partial y \partial t^2} \right) + I_2 \left( \frac{\partial^4 W}{\partial x^2 \partial t^2} \right) = F_c^*(t), \quad (50)$$

where

$$(I_0, I_1, I_2) = \int_{-h}^h \rho(1, z, z^2) dz. \tag{51}$$

Substituting Eqs. (12) to (17) into Eqs. (48) to (50), the governing equations can be written as follow

$$\begin{aligned} & \frac{\partial}{\partial x} \left( A_{11} \frac{\partial U}{\partial x} - A_{11z} \frac{\partial^2 W}{\partial x^2} + A_{12} \frac{\partial V}{\partial y} - A_{12z} \frac{\partial^2 W}{\partial y^2} \right) \\ & + \frac{\partial}{\partial y} \left( A_{44} \frac{\partial U}{\partial y} + A_{44} \frac{\partial V}{\partial x} - 2A_{44z} \frac{\partial^2 W}{\partial x \partial y} \right) \\ & = \left[ I_0 \frac{\partial^2 U}{\partial t^2} - I_1 \frac{\partial^3 W}{\partial x \partial t^2} \right], \end{aligned} \tag{52}$$

$$\begin{aligned} & \frac{\partial}{\partial x} \left( A_{44} \frac{\partial U}{\partial y} + A_{44} \frac{\partial V}{\partial x} - 2A_{44z} \frac{\partial^2 W}{\partial x \partial y} \right) \\ & + \frac{\partial}{\partial y} \left( A_{21} \frac{\partial U}{\partial x} - A_{21z} \frac{\partial^2 W}{\partial x^2} + A_{22} \frac{\partial V}{\partial y} - A_{22z} \frac{\partial^2 W}{\partial y^2} \right) \\ & = \left[ I_0 \frac{\partial^2 V}{\partial t^2} - I_1 \frac{\partial^3 W}{\partial y \partial t^2} \right], \end{aligned} \tag{52}$$

$$\begin{aligned} & \frac{\partial^2}{\partial x^2} \left( A_{11z} \frac{\partial U}{\partial x} - B_{11} \frac{\partial^2 W}{\partial x^2} + A_{12z} \frac{\partial V}{\partial y} - B_{12} \frac{\partial^2 W}{\partial y^2} \right) \\ & + 2 \frac{\partial^2}{\partial x \partial y} \left( 2A_{44z} \frac{\partial U}{\partial y} + 2A_{44z} \frac{\partial V}{\partial x} - 2B_{44} \frac{\partial^2 W}{\partial x \partial y} \right) \\ & + \frac{\partial^2}{\partial y^2} \left( A_{21z} \frac{\partial U}{\partial x} - B_{21} \frac{\partial^2 W}{\partial x^2} + A_{22z} \frac{\partial V}{\partial y} - B_{22} \frac{\partial^2 W}{\partial y^2} \right) \\ & = I_0 \left( \frac{\partial^3 W}{\partial t^2} \right) + I_1 \left( \frac{\partial^3 U}{\partial x \partial t^2} + \frac{\partial^3 V}{\partial y \partial t^2} \right) \\ & - I_2 \left( \frac{\partial^4 W}{\partial x^2 \partial t^2} + \frac{\partial^4 W}{\partial y^2 \partial t^2} \right) + F_c^*(t), \end{aligned} \tag{54}$$

**6. Numerical solution**

The DQM approximates the partial derivative of a function  $F$ , with respect to two spatial variables ( $x$  and  $y$ ) at a given discrete point  $(x_i, y_i)$ , as a weighted linear sum of the function values at all discrete points chosen in the solution domain ( $0 < x < L, 0 < y < b$ ) with  $N_x \times N_y$  grid points along  $x$  and  $y$  axes, respectively. Then, the  $n^{\text{th}}$ -order partial derivative of  $F(x, y)$  with respect to  $x$ , the  $m^{\text{th}}$ -order partial derivative of  $F(x, y)$  with respect to  $y$  and the  $(n + m)^{\text{th}}$ -order partial derivative of  $F(x, y)$  with respect to both  $x$  and  $y$  is expressed discretely at the point  $(x_i, y_i)$  as (Al-Furjan *et al.* 2022a, b, c, d, e, f, 2021a, b, c, 2020, Kolahchi *et al.* 2016, 2017a, b, 2018, 2020a, b, 2021, 2022)

$$\frac{d^n F(x_i, y_j)}{dx^n} = \sum_{k=1}^{N_x} A_{ik}^{(n)} F(x_k, y_j) \tag{26}$$

$$\frac{d^m F(x_i, y_j)}{dy^m} = \sum_{l=1}^{N_y} B_{jl}^{(m)} F(x_i, y_l) \tag{27}$$

$$\frac{d^{n+m} F(x_i, y_j)}{dx^n dy^m} = \sum_{k=1}^{N_x} \sum_{l=1}^{N_y} A_{ik}^{(n)} B_{jl}^{(m)} F(x_k, y_l), \tag{28}$$

where  $A_{ik}^{(n)}$  and  $B_{jl}^{(m)}$  are the weighting coefficients which may be defined as

$$A_{ij}^{(1)} = \begin{cases} \frac{M(x_i)}{(x_i - x_j)M(x_j)} & \text{for } i \neq j, \quad i, j = 1, 2, \dots, N_x \\ -\sum_{\substack{j=1 \\ i \neq j}}^{N_x} A_{ij}^{(1)} & \text{for } i = j, \quad i, j = 1, 2, \dots, N_x \end{cases} \tag{58}$$

$$B_{ij}^{(1)} = \begin{cases} \frac{P(y_i)}{(y_i - y_j)P(y_j)} & \text{for } i \neq j, \quad i, j = 1, 2, \dots, N_y, \\ -\sum_{\substack{j=1 \\ i \neq j}}^{N_y} B_{ij}^{(1)} & \text{for } i = j, \quad i, j = 1, 2, \dots, N_y \end{cases} \tag{59}$$

where  $M$  and  $P$  are Lagrangian operators which are defined as

$$M(x_i) = \prod_{\substack{j=1 \\ j \neq i}}^{N_x} (x_i - x_j) \tag{60}$$

$$P(y_i) = \prod_{\substack{j=1 \\ j \neq i}}^{N_y} (y_i - y_j) \tag{61}$$

The weighting coefficients for the second, third and fourth derivatives are determined via matrix multiplication,

$$A_{ij}^{(n)} = n \left( A_{ii}^{(n-1)} A_{ij}^{(1)} - \frac{A_{ij}^{(n-1)}}{(x_i - x_j)} \right) \tag{62}$$

$$B_{ij}^{(m)} = m \left( B_{ii}^{(m-1)} B_{ij}^{(1)} - \frac{B_{ij}^{(m-1)}}{(y_i - y_j)} \right) \tag{63}$$

Using the following rule, the distribution of grid points in domain is calculated as

$$x_i = \frac{L}{2} \left[ 1 - \cos \left( \frac{i-1}{N_x-1} \pi \right) \right] \quad i = 1, \dots, N_x \tag{64}$$

$$y_i = \frac{b}{2} \left[ 1 - \cos \left( \frac{i-1}{N_y-1} \pi \right) \right] \quad i = 1, \dots, N_y \tag{65}$$

Using DQM, the motion equations for low-velocity impact of structure can be expressed in matrix form as

$$[M]\{\ddot{\chi}\} + [C_e]\{\dot{\chi}\} + [K]\{\chi\} = \{Q\} \tag{66}$$

where  $[M]$ ,  $[k]$  and  $[C_e]$  are the mass, stiffness and damp matrixes, respectively,  $\{\chi\}$  is the dynamic vector,  $Q$  is the dynamic load vector. Defining the second and first time derivatives using Teoplitz matrices as follows

$$\begin{cases} a_{11} = 0, \\ a_{i,1} = (-1)^{i-1} \cot \left( \frac{\pi(i-1)}{n} \right), \\ a_{1,j} = (-1)^{n-j+1} \cot \left( \frac{\pi(n-j+1)}{n} \right), \\ a_{i+1,j+1} = a_{ij}, \end{cases} \quad i, j = 2, 3, 4, \dots, n, \quad \tilde{D}_i^1 = 2\pi [a_{ij}], \tag{67}$$

$$\begin{cases} b_{11} = -\frac{n^2}{12} - \frac{1}{6}, \\ b_{i,i} = \frac{(-1)^{i-1}}{2 \sin^2\left(\frac{\pi(i-1)}{n}\right)}, \\ b_{i,j} = \frac{(-1)^{n-j+1}}{2 \sin^2\left(\frac{\pi(n-j+1)}{n}\right)}, \\ b_{i+1,j+1} = b_{ij}, \end{cases} \quad i, j = 2, 3, 4, \dots, n, \quad \tilde{D}_t^2 = (2\pi)^2 [b_{ij}]. \quad (68)$$

Eq. (77) can be written as

$$([D_t^2 \otimes M] + [D_t^1 \otimes C_e] + [I_t \otimes K])\chi = [Q], \quad (69)$$

where  $\otimes$  notes the Kronecker product and  $I_t$  is unit matrix. Finally, solving above equation yields to deflection and contact force of structure which are discussed in next section.

### 7. Numerical Results and discussion

In order to show the effect of different parameters on the dynamic deflection and contact force of the structure, the concrete plate with Young’s modulus of  $E_c = 20 \text{ GPa}$  and Poisson’s ratio of  $\nu_c = 0.2$  is considered. The plate is reinforced by carbon nanoparticles by Young’s modulus of  $E_n = 1 \text{ TPa}$  and Poisson’s ratio of 0.3.

At the first, we want to discuss about the standard deviation of the results. For example, the data of Figs. 3 and 4 is assumed. For computing the standard deviation, we should calculate:

- 1-Average of the deflection and contact force
- 2- The square of the difference of each data value with the mean
- 3- The variance of the data, which is the mean square of the difference between the data and its mean
- 4- The root of the variance as the standard deviation of the data

The standard deviation for the deflection and contact force are 0.5517 and 0.4920, respectively which show a good and uniform distribution of data.

The effect of velocity and mass of impactor on the histories of contact force and deflection is demonstrated in Figs. 3 and 4, respectively. As can be seen the contact force and the central deflection of the concrete plate increase with increasing impact velocity of impactor while the contact duration decreases as the impact velocity of impactor increases. The reason is that the higher impact velocity of impactor which accompanies a higher impact energy, requires a larger deflection and accompanying contact force to dissipate it.

The impactor height effect on the histories of contact force and deflection is demonstrated in Figs. 5 and 6, respectively. As can be seen the contact force and the central deflection of the concrete plate increase with increasing impact velocity of impactor while the contact duration decreases as the impact velocity of impactor increases. The reason is that the higher impact velocity of impactor which accompanies a higher impact energy,

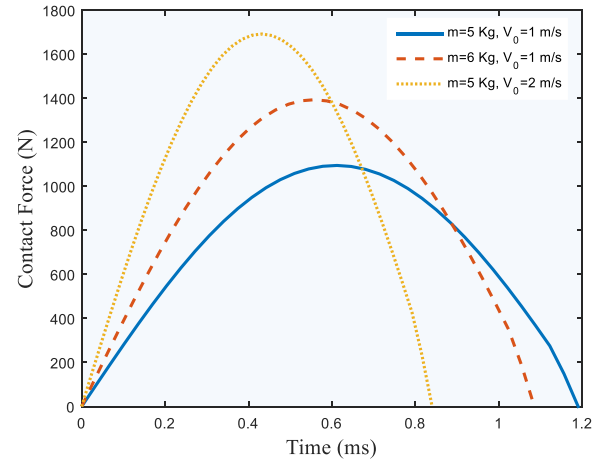


Fig. 3 The impact velocity and mass effect on the history of contact force

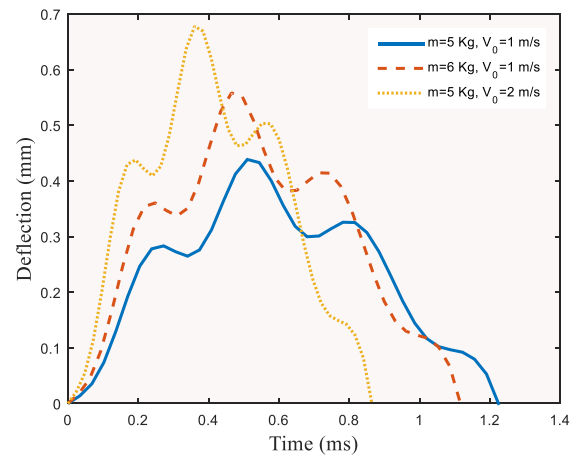


Fig. 4 The impact velocity and mass effect on the history of dynamic deflection

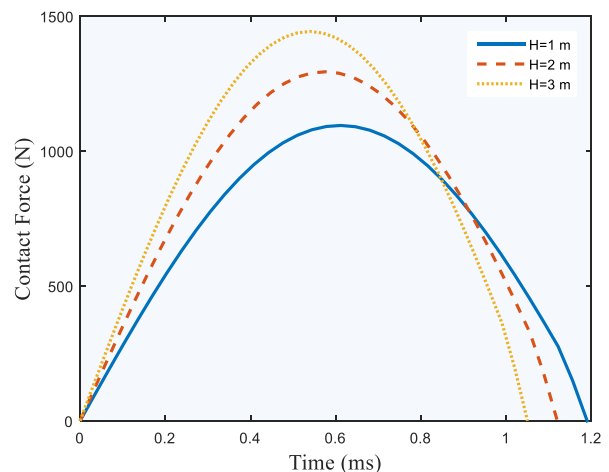


Fig. 5 The impactor height effect on the history of contact force

requires a larger deflection and accompanying contact force to dissipate it.

Figs. 7 and 8 show the effects of nanoparticle volume percent on the contact force and deflection histories of

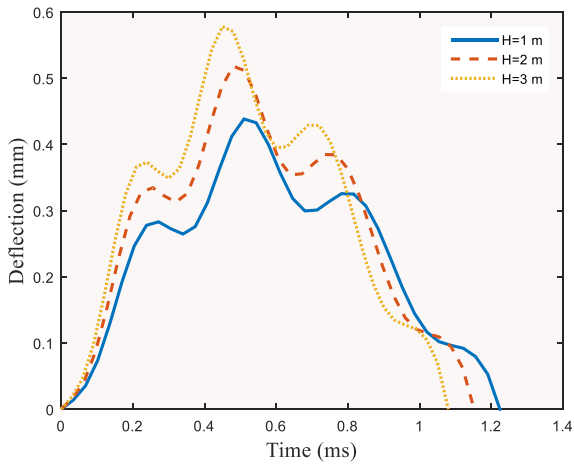


Fig. 6 The impactor height effect on the history of dynamic deflection

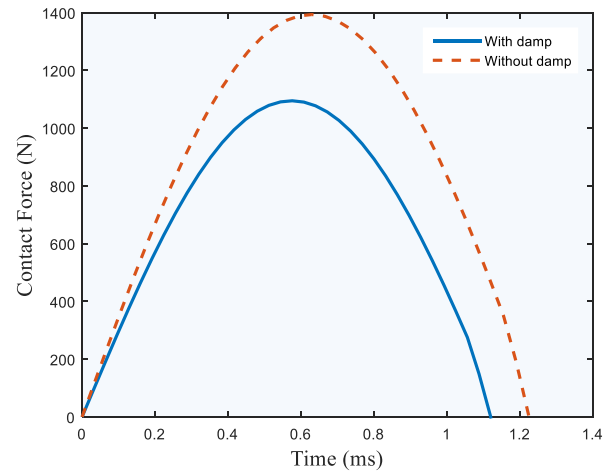


Fig. 9 The structural damping effect on the history of contact force

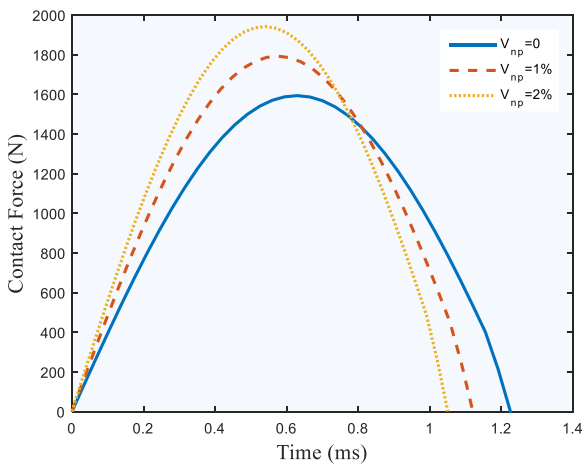


Fig. 7 The nanoparticle volume percent effect on the history of contact force

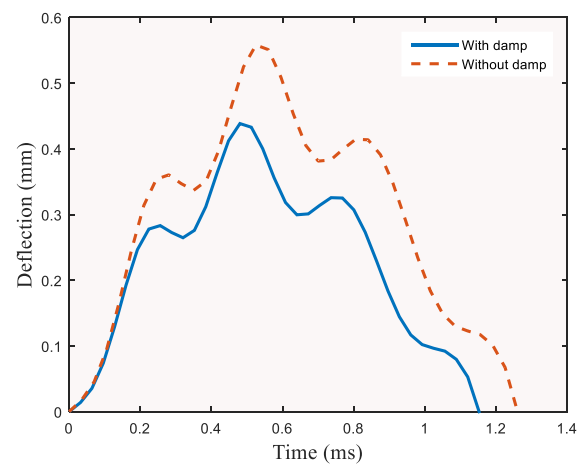


Fig. 10 The structural damping effect on the history of dynamic deflection

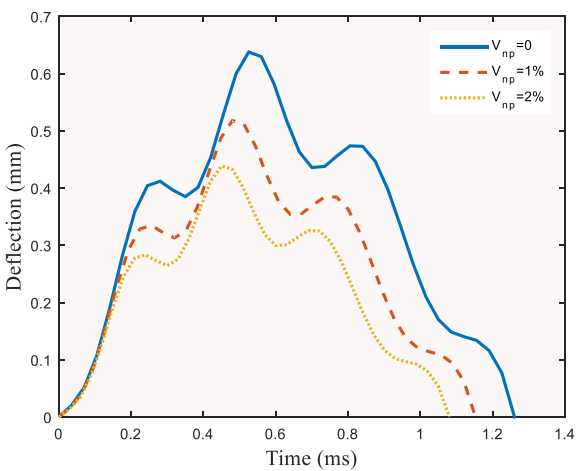


Fig. 8 The nanoparticle volume percent effect on the history of dynamic deflection

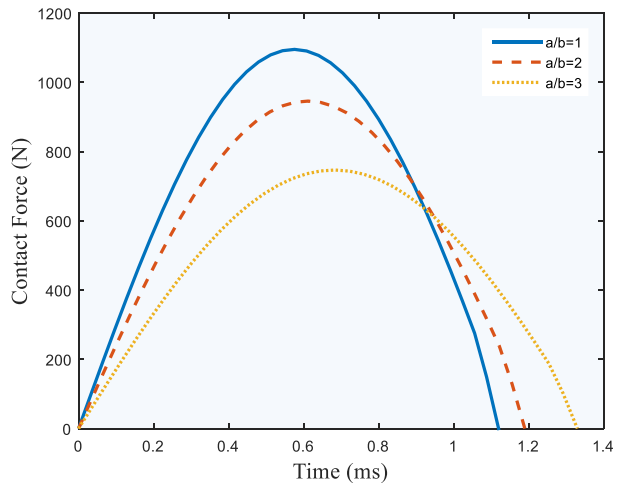


Fig. 11 The length to width ratio of plate effect on the history of contact force

structure, respectively. These figures show that with increasing the nanoparticle volume percent, the maximum contact force decreases slightly. It is since with enhancing the nanoparticle volume percent, the stiffness of the

structure is increased. In order to quantitative study, for the concrete plate without nanoparticles, the maximum deflection and contact force are 0.651 mm and 1580 N while these values are 0.43 mm and 1910 N, respectively

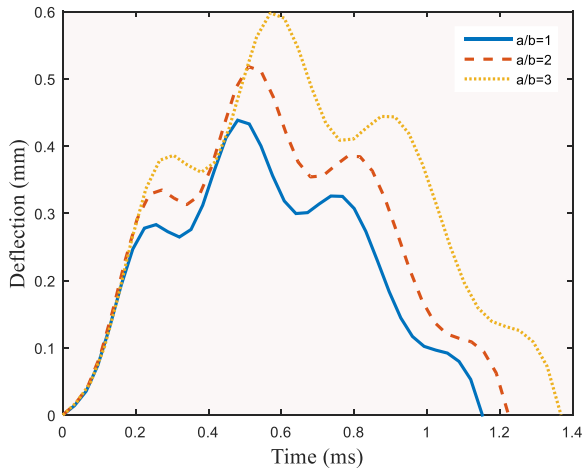


Fig. 12 The length to width ratio of plate effect on the history of dynamic deflection

for volume percent of nanoparticles equal to 2%. In other words, with increasing the the volume percent of nanoparticle, the maximum deflection and contact force are decreased 34% and increased 21%, respectively.

Figs. 9 and 10 depict, respectively contact force and central deflection histories of structure for cases including without structural damping and with structural damping. As can be seen from these figures, considering structural damping decreases the contact force and deflection of the structure.

The effect of the length to width ratio of plate on the contact force and deflection histories is demonstrated in Figs. 11 and 12, respectively. The figure shows that the contact force and the central deflection of the structure increases and the contact time decreases with decreasing length to width ratio of plate. This behavior is due to the decrease of the stiffness with increasing length to width ratio of plate.

## 8. Conclusions

In this article, low velocity impact response of the concrete plates was presented applying a new equivalent TDOF spring–mass–damper model. The concrete plate is reinforced by carbon nanoparticles which the effective material proprieties are obtained by mixture’s rule. Based on CPT, the motion equations were derived using energy method and Hamilton’s principle. Exact solution is applied for obtaining the dynamic deflection and contact force of system so that the effects of the of mass, velocity and height of impactor, volume percent of nanoparticles, structural damping and geometrical parameters of structure are shown on the dynamic deflection and contact force of system were considered. Results indicated considering structural damping decreases the contact force and deflection of the structure. In addition, the contact force and the central deflection of the structure increases and the contact time decreases with decreasing length to width ratio of plate. With increasing the the volume percent of nanoparticle, the maximum deflection and contact force were decreased 34%

and increased 21%, respectively. Furthermore, the contact force and the central deflection of the structure increase with increasing impact velocity of impactor while the contact duration decreases as the impact velocity of impactor increases.

In this paper, low velocity impact load is assumed with relatively high masses and low impact velocities. In the future works, the high velocity impact of this structure can be novel topic for researchers. High velocity impact loads from scenarios like particle impact, involving comparably small masses and high. It is also noted that for high velocity impact, the stress relations, the elasto-plastic formulation should assumed (Žmindák *et al.* 2016) and for the impact force we should use from a new relations (Alonso and Solis, 2021).

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