

Nonlinear intelligent control systems subjected to earthquakes by fuzzy tracking theory

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Abstract. Uncertainty of the model, system delay and drive dynamics can be considered as normal uncertainties, and the main source of uncertainty in the seismic control system is related to the nature of the simulated seismic error. In this case, optimizing the management strategy for one particular seismic record will not yield the best results for another. In this article, we propose a framework for online management of active structural management systems with seismic uncertainty. For this purpose, the concept of reinforcement learning is used for online optimization of active crowd management software. The controller consists of a differential controller, an unplanned gain ratio, the gain of which is enhanced using an online reinforcement learning algorithm. In addition, the proposed controller includes a dynamic status forecaster to solve the delay problem. To evaluate the performance of the proposed controllers, thousands of ground motion data sets were processed and grouped according to their spectrum using fuzzy clustering techniques with spatial hazard estimation. Finally, the controller is implemented in a laboratory scale configuration and its operation is simulated on a vibration table using cluster location and some actual seismic data. The test results show that the proposed controller effectively withstands strong seismic interference with delay. The goals of this paper are towards access to adequate, safe and affordable housing and basic services, promotion of inclusive and sustainable urbanization and participation, implementation of sustainable and disaster-resilient buildings, sustainable human settlement planning and manage. Simulation results is believed to achieved in the near future by the ongoing development of AI and control theory.

Keywords: damage identification; fuzzy monitoring; Kalman filter; measurement problems; nonlinear hysteresis equation; unknown inputs

1. Introduction

Protecting buildings from natural disasters has always been a major concern in earthquake zones. The main goal of the building management system is to protect the building from external influences such as wind, earthquake and pollution by reducing the vibration transmitted to the building. These methods have been the subject of extensive research in recent years as buildings have become thicker and more flexible. (e.g., Safa 2016, Shariat *et al.* 2018). Especially in system engineering, many methods have been developed to ensure the reliability and performance of non-linear systems. For example, Casciati (1997), Ning *et al.* (2024), Yin *et al.* (2022, 2023), Yu *et al.* (2022), Casciati and Faravelli (2009) and Casciati *et al.* (2014) propose an AI-based approach for engineering applications with non-linear systems.

Structural management methods can be divided into passive, passive and active groups. From conventional isolation systems to adjustable mass dampers (TMD) and

liquid column adjustment, passive system dampers use passive elements to dry out vibrations caused by external loads. The semi-active control system, on the other hand, controls the stiffness or stiffness of the system to reduce the vibration caused. Finally, the active structure control system includes actuators that control the behavior of the structure in response to external stimuli. Active structure management systems are well established among structural management systems (Wang *et al.* 2022, 2024, Gao *et al.* 2024, Zhang *et al.* 2023, 2024, Chen *et al.* 2022, 2023a, b).

The main principle of the active antenna array management system is its controller. Over the past two decades, several management algorithms have been used to manage these systems. One of the challenges in designing a functional structure management system is to identify the ambiguity of the uncertainty in forward-looking excitement that implies dynamics. Arrival time delays are another source of uncertainty that may be related to the dynamics of the process, processing time, or network transfer time. Robust control testing is performed in many functional structure control programs, which are common methods for resolving uncertainties. Ballandin and Kogan proposed reliable monitoring of H_∞ for seismic resistance of high-rise buildings using linear matrix inequalities. Fuzzy logic control (FLC) is also widely used for active / semi-active control of insecure structural systems due to its robustness.

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This controller is ideal for managing building systems where the quality of the building is constantly changing due to the number of occupants and different types of properties. In addition, local damage to structural elements due to external loads can alter the rigidity of the structure. (Dai *et al.* 2023, Deng *et al.* 2024, Xiao *et al.* 2023a, b, c, Xu *et al.* 2022a, b, Du and Wang 2024, Fang *et al.* 2024a, b).

In addition, the H_∞ management approach is very useful for building strong management. These hardness parameters can be interpreted in part as H_∞ of the transfer function below a given value. This article presents an online management framework for functional structure management systems that simultaneously take into account earthquake uncertainties and time delays. For this purpose, we use the concept of Reinforcement Learning (RL) algorithm for online training of dynamic processors with unplanned controllers. In addition, the proposed controller has a built-in state predictor to calculate the delay time. The control system is implemented in a laboratory scale configuration and its performance is verified through extensive shaker testing under a combination of possible seismic events at a given location.

The rest of the section is organized as follows: First, a structural model and a mathematical model are presented. Control methods and control configurations will be described later. Next, the structure of the seismic fault is considered. Finally, we report and discuss the implementation of control algorithms and test results.

2. Criteria and methods

In this study, the optimal performance of a 10-storey building was considered. The Fig. 1 shows a schematic representation of a structure modeled on a shear frame with 10 degrees of freedom (DOF). Eq. (1) gives the dynamics of motion control of a model in matrix form (Wu *et al.* 2006, Khalatbarisoltani *et al.* 2019)

$$\begin{aligned} M\ddot{X} + C\dot{X} + KX &= -Mr\ddot{x}_g(t) + Du(t) \\ C &= a_0M + a_1K; \quad a_0 = \frac{2\xi W_1 W_3}{W_1 + W_3}; \quad a_1 = \frac{2\xi}{W_1 + W_3} \\ M &= \begin{bmatrix} m_1 & \cdots & 0 \\ \vdots & \ddots & \vdots \\ 0 & \cdots & m_n \end{bmatrix}; \quad C = \begin{bmatrix} c_1 + c_2 & & \\ & \ddots & \\ & & 0 \end{bmatrix} \\ &\quad \dots \quad K = \begin{bmatrix} k_1 + k_2 & \cdots & 0 \\ \vdots & \ddots & \vdots \\ 0 & \cdots & k_n \end{bmatrix} \end{aligned} \quad (1)$$

where M is the mass matrix, K is the spring matrix, $u(t)$ is the external matrix to control or drive the active energy, r is the voltage coefficient matrix, and D is the control input coefficient matrix. In addition, C is the Rayleigh damping matrix and represents the first and third structural modes, x is the displacement vector and \dot{x} is the ground velocity later. Using the existing weight, hardness and drying matrices of the model, the equations of motion can be written in the following state spaces

$$\begin{aligned} \dot{x}(t) &= AX(t) + BU(t) + EF(t); \\ A &= \begin{bmatrix} 0 & 1 \\ -M^{-1}K & -M^{-1}C \end{bmatrix}_{n \times n}; \\ B &= \begin{bmatrix} 0 \\ M^{-1} \end{bmatrix}_{n \times m}; \quad E = \begin{bmatrix} 0 \\ M^{-1} \end{bmatrix}_{n \times 1} \end{aligned} \quad (2)$$

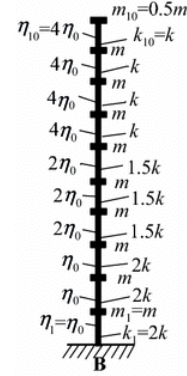


Fig. 1 Schematic representation of a structure modeled on a cross-sectional frame with 10 degrees of freedom (DOF)

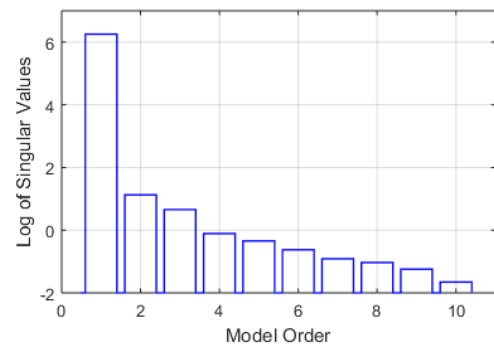


Fig. 2 Hankel singular value estimate for prototype with 10 degrees of freedom

Considering how basic the prototype in this work is reduced to a model with two levels of freedom. The Fig. 2 shows the calculated Hankel singular coefficient of the original model with 10 degrees of freedom. The singular value is a measure of the energy of the corresponding state

and its contribution to the process of reaction. As you can see, the individual values assigned to scenarios 1-4 affect the response variables more than any other scenario. This result is consistent with the fact that the fundamental frequency has the greatest influence on the structural variation of the response. Table 2 also shows the natural frequencies of the prototypes. According to Table 2, the third mode is higher than the second. Based on the above considerations, we decided to abandon the third and higher order stages and create a reduced 2DOF version of the original system for on-board testing. A simple system is created with two levels of freedom, the natural frequency and its form corresponding to the original structure. For this, the Rayleigh-Ritz method is used. The test

Table 1 Selected the first five natural frequencies of the sample structure with 10 degrees of freedom

Modal no.	Natural frequency	
	Rad/s	rpm
1	1323.2	12,636
2	1323.2	12,636
3	1649.1	15,748
4	2059.0	19,662
5	2245.1	21,439

Scale Factor

A **scale factor** is the number by which all the components of an object are multiplied in order to create a proportional enlargement or reduction.

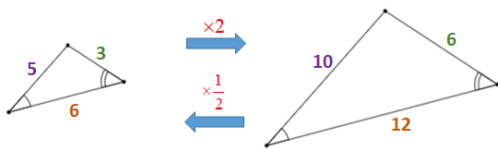


Fig. 3 Reduced model dimensions by the techniques of scale factor description

configuration is based on a simple model. Use the Ibrahim Time Domain (ITD) method to determine the modular parameters of your model, such as natural frequency and waveform. In addition, the integrated test configuration is checked against the test protocol. We compare the eigenfrequencies of the prototype, the simple sample, and the experimental setup. It can be seen that the modular parameters of the experimental configuration are in good agreement with the initial configuration. Fig. 3 shows the experimental setup after a simple procedure.

Eq. (3) shows the equation of motion for this model.

$$\begin{bmatrix} \dot{x}_1 \\ \dot{x}_2 \\ \dot{x}_3 \\ \dot{x}_4 \end{bmatrix} = \begin{bmatrix} 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 \\ -(k_1 + k_2) & k_2 & -(c_1 + c_2) & 0 \\ m_1 & m_1 & m_1 & 0 \\ k_2 & -k_2 & c_2 & 0 \\ m_2 & m_2 & m_2 & 0 \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \\ x_3 \\ x_4 \end{bmatrix} + \begin{bmatrix} 0 & 0 \\ 0 & 0 \\ -1 & 0 \\ -1 & 1 \\ -1 & \frac{1}{m_2} \end{bmatrix} [\ddot{x}_g u(t)], \tag{3}$$

3. Extended Kalman filter based fuzzy theory

On the Figs. 4-5 we will learn how to apply obscure management functions and membership to similar virtual systems. In addition, RL algorithms are methods for autonomous design that are closely related to the environment. Learning reinforcement agents are often advised to approach their good manners with immediate rewards. For example, it is used in some design features of controls.

Learning strategies fall into three categories: controlled learning, uncontrolled learning, and reinforced learning (RL). Artificial neural networks and GAs have been studied in several previous studies in the field of active structure management, but RL, although its properties have not been scaled to a sufficient level for parameter learning. For example, linear artificial neural network controllers and joint controllers of the cerebellar model are common approaches to structural control.

RL algorithms solve problems in which an autonomous agent understands a specific situation from its point of view and performs a specific action to achieve a goal. When an agent acts in nature, he or she is rewarded or punished depending on the action and the circumstances. This article uses the Q-learning algorithm.

We then create dynamic structure management issues as the Markov Decision Process (MDP) for RL systems. MDP is a mathematical model for sequencing decision problems. In this paper, we create a system management problem that acts as MDP, so the RL algorithm with MDP is described by the 5th order (S, A,, γR T_{xa}). Where S is the order of s. a = [k_p(m), k_a(n)], is a set of obscure rules for the acquisition of a PD controller divided into levels from m = 1 to D and from n = 1 to DD. Except for individual variables, the uncertainty of the model is not considered in this paper T_{xa}. In this case, the controllers are rewarded based on their response to the architecture, so the legal basis of the

controllers is updated regularly. On the picture. 6 Introduce MDP with current architectural management strategies. In this diagram, they are defined as states or actions.

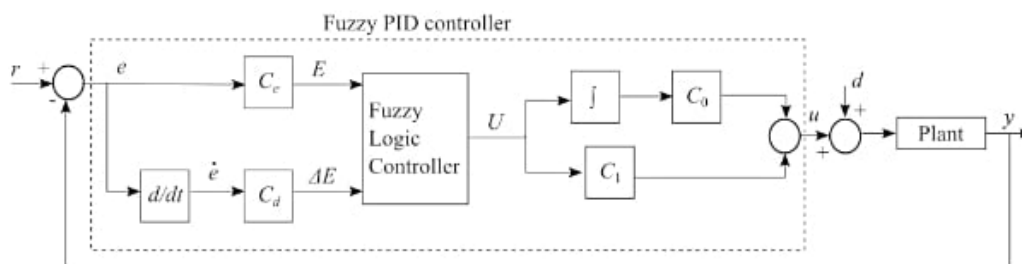


Fig. 4 Controller design for the fuzzy theory

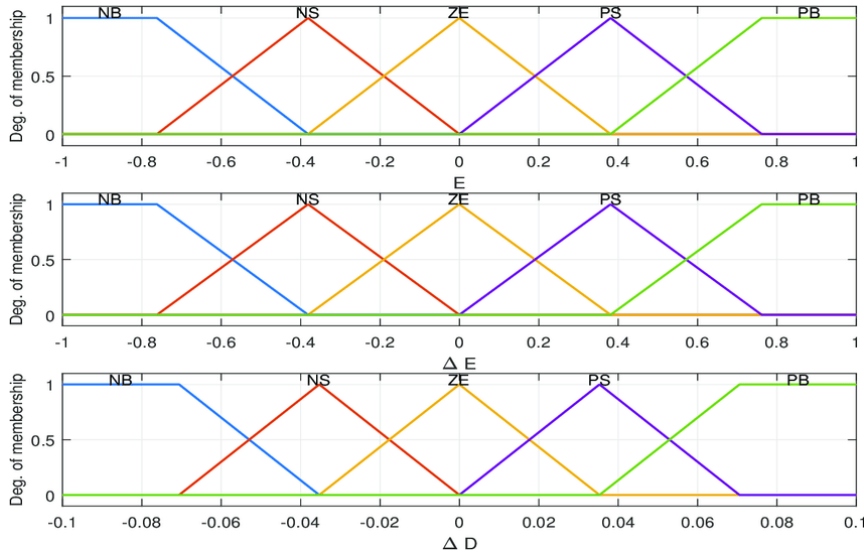


Fig. 5 Inputs and output fuzzy membership functions: (a) displacement of top story; (b) velocity of top story; (c) force

The result of the Q learning algorithm is the Q value, which represents the behavior of the active management system of the structure.

The AQ Q value (s, a) is adjusted periodically according to Eq. (4).

$$Q(s, a) = Q(s, a) + \alpha(r + \gamma \max_{a' \in A} Q(s', a') - Q(s, a)) \quad (4)$$

Where are $\alpha \in [0,1]$ the study rates and $\gamma \in [0,1]$; The reward function is also defined as Eq. (5).

Here, m_i 10 single-axis accelerometers were placed on the ground to evaluate the basic displacement of the main structure analyzed for the volume of the second floor and the corresponding acceleration. \ddot{x}_i The accelerometer can be integrated in accordance with the data of Eq. (6). Base changes are calculated online. Please note that the basic changes calculated in this way may not be very accurate due to differences in the number of people and the type of furniture.

Also $\alpha_1, i = 1, \dots, 5$ is a normal variable. The controller input calculated based on the specification of the PD controller is defined in Eq. (7).

$$R = -1 \left(\alpha_1 \frac{|x_{Top}(i)|}{|x_{Topmax}|} \right. \\ \left. \alpha_2 \frac{|x_{Top}(i)|}{x_{Topmax}} \right. \\ \left. \alpha_3 \frac{|Force(i)|}{|Forcemax|} \right. \\ \left. \alpha_4 \frac{|Base\ Shear(i)|}{|Base\ Shear_{max}|} \right. \\ \left. \alpha_5 \frac{|Drift(i)|}{|Drift_{max}|} \right) \quad (5)$$

$$\text{Base shear} = \sum_{i=1}^{i=10} m_i \ddot{x}_i, \quad (6)$$

$$u(t) = -F_{kp}(x(t) - x_d(t)) - F_{kd}(\dot{x}(t) - \dot{x}_d(t)), \quad (7)$$

Where is also F_{kp} the increase in control set by the controller, maintenance, standby $x(t)$ and F_{kd} the above bias. $x_d(t)$ Desired displacement of the surface layer. Eq. (7) is rewritten as Eq. (8). This allows for zero expected returns, which is a good case for a structured response.

$$u(t) = -F_{kp}x(t) - F_{kd}\dot{x}(t) \quad (8)$$

Winning rules are calculated by two independent FLCs. Its legal basis is regularly updated with the proposed RL approach. The stability of the dynamic system is estimated by the sign of the eigenvalues of matrix A as follows

$$\det(sI - A) = s^4 + b_1s^3 + b_2s^2 + b_3s^1 + b_4, \quad (9)$$

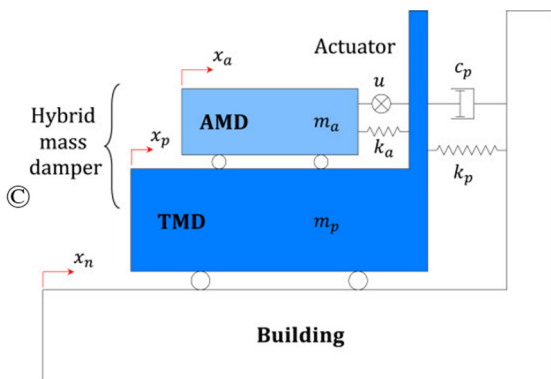


Fig. 6 MDP design criteria for actively managed buildings

Neural networks can be written

$$X(t) = \Psi^S(W^S\Psi^{S-1}(W^{S-1}\Psi^{S-2}(\dots\Psi^2(W^2\Psi^1(W^1\Lambda(t))))\dots)), \tag{10}$$

Where $\Lambda^T(t) = [X^T(t) \ U^T(t)]$, as $X^T(t) = [x_1(t) \ x_2(t) \ \dots \ x_6(t)]$ in the S range, each layer $R^\sigma(\sigma = 1, 2, \dots, S)$ contains neurons and $x_1(t) \sim x_\delta(t)$ is an input variable. $u_1(t) \sim u_m(t)$ Symbolic Weights W^σ Matrix Layer σ^{th} The transfer function vector σ^{th} is defined $\Psi^\sigma(v) = [T(v_1) \ T(v_2) \ \dots \ T(v_{R^\sigma})]^T$ for this layer. ($\sigma = 1, 2, \dots, S$).

The NNDI system can be described in state spatial representation (see Bai *et al.* 2021, Cao *et al.* 2023, Hu *et al.* 2023, Li *et al.* 2022, 2023, 2024, Liang *et al.* 2024a, b) as follows

$$Y(t) = A(a(t))Y(t),$$

$$A(a(t)) = \sum_{i=1}^{\gamma} h_{\gamma}(a(t))\bar{A}_i,$$

Where r is a positive integer; is a vector that shows $a(t)$ its dependence on the element $h_i(\cdot)$, i.e., $h_i(a(t)) = h_i(a_1(t), a_2(t), \dots, a_n(t))$, $a(t) = [a_1(t), a_2(t), \dots, a_n(t)]^T$ (usually $a(t)$ parallel to the state vector $X(t)$ is $\bar{A}_i(i = 1, 2, \dots, r)$ a fixed matrix $Y(t) = [y_1(t) \ y_2(t) \ \dots \ y_j(t)]^T$.

By interpolation we get

$$X(t) = \left[\sum_{\zeta^S=1}^2 h_{\zeta^S}(t) G_{\zeta^S} \left(W^S \left[\sum_{\zeta^1=1}^2 h_{\zeta^1}(t) G_{\zeta^1}(W^1\Lambda(t)) \right] \right) \right]$$

$$= \sum_{\zeta^S=1}^2 \dots \sum_{\zeta^2=1}^2 \sum_{\zeta^1=1}^2 h_{\zeta^S}(t) \dots h_{\zeta^2}(t) h_{\zeta^1}(t) G_{\zeta^S} W^S \dots G_{\zeta^2} W^2 G_{\zeta^1} W^1 \Lambda(t) = \sum_{\Omega^\sigma} h_{\Omega^\sigma}(t) E_{\Omega^\sigma} \Lambda(t) \tag{11}$$

Where $\sigma = 1, 2, \dots, S$, $h_{q^\sigma}(t) \in [0 \ 1]$,

$$\sum_{q^\zeta=1}^2 h_{q^\zeta}^\sigma(t) = 1; \ \zeta = 1, 2, \dots, R^\sigma;$$

$$E_{\Omega^\sigma} \equiv G_{\zeta^S} W^S \dots G_{\zeta^2} W^2 G_{\zeta^1} W^1,$$

$$\sum_{\Omega^\sigma} h_{\Omega^\sigma}(t) \equiv \sum_{\zeta^S=1}^2 \dots \sum_{\zeta^2=1}^2 \sum_{\zeta^1=1}^2 h_{\zeta^S}(t) \dots h_{\zeta^2}(t) h_{\zeta^1}(t).$$

$$\sum_{\zeta^\sigma} h_{\zeta^\sigma}(t) = \sum_{q_1^\sigma=1}^2 \sum_{q_2^\sigma=1}^2 \dots \sum_{q_{R^\sigma}^\sigma=1}^2 h_{q_1^\sigma}(t) h_{q_2^\sigma}(t) \dots h_{q_{R^\sigma}^\sigma}(t).$$

The dynamics of the NN model can be rewritten as NNNDI

$$\dot{X}(t) = \sum_{i=1}^{\gamma} h_i(t) \bar{E}_i \Lambda(t), \tag{12}$$

Where $h_i(t) \geq 0; \sum_{i=1}^{\gamma} h_i(t) = 1$; r is a positive integer; And \bar{E}_i is a fixed matrix with related dimensions E_{Ω^σ} . The NNNDI view can be further restructured as follows

$$\dot{X}(t) = \sum_{i=1}^{\gamma} h_i(t) \{A_i X(t)\}, \tag{13}$$

where A_i These are the parts E_i that match the parts $\Lambda(t)$.

According to the above NN-based system modeling scheme, the nonlinear system can be approximated by the NNNDI representation. The NNNDI representation follows the same rules as the TS machine learning fuzzy model, combining the flexibility of fuzzy logic theory of machine learning and the rigorous mathematical analysis tools of linear model theory into one model. To test the robustness of the TLP algorithm, we recall the obscure machine learning pattern and TS robust analysis. First, the i -th rule of the TS fuzzy machine learning model, which represents the structural model, can be specified as

$$\text{Me } x_1(t): \dots \text{Yes, } M_{i1} \text{ yes } x_p(t). M_{ip}$$

$$\text{Eat } \dot{x}_j(t) + \sum_{k=1}^{N_j} A_{ikj} x_j(t - \tau_{kj}) + B_{ij} u_j(t) \tag{14}$$

in which $i = 1, 2, \dots, r$ control numbers; $X(t)$ is the state vector. $M_{ip}(p = 1, 2, \dots, g)$ Fuzzy learning set of $x_1(t) \sim x_p(t)$ variable factor machine.

4. Fuzzy control design

In the following, a stability criterion is proposed to guarantee the asymptotic stability of the nonlinear multiple time-delay large-scale system N. Prior to examination of asymptotic stability, a useful concept is given below.

Lemma 1: For any matrices X and Y with appropriate dimensions, we have $X^T Y + Y^T X \leq \xi X^T X + \xi Y^T Y$ where ξ is a positive constant.

Theorem 1: The nonlinear multiple time-delay large-scale system N is asymptotically stable, if there exist positive constants β, α_j, k_j and $\rho_j, j = 1, 2, \dots, J$ and the feedback gains K_{ij} 's are chosen to satisfy

$$\bar{\lambda}_j = \max_k \lambda_m(\bar{Q}_{kj} < 0) \text{ for } k = 1, 2, \dots, N_j$$

$$\lambda_{ij} \equiv \lambda_m(Q_{ij}) > 0 \text{ for } i = 1, 2, \dots, r_j$$

$$\lambda_{ifj} \equiv \lambda_m(Q_{ifj}) > 0 \text{ for } i < f \leq r_j$$

or

$$A_j = \begin{bmatrix} -\bar{\lambda}_j & 0 & 0 & \dots & 0 \\ 0 & \lambda_{1j} & \frac{1}{2} \lambda_{12j} & \dots & \frac{1}{2} \lambda_{1r_j j} \\ 0 & \frac{1}{2} \lambda_{12j} & \lambda_{2j} & \dots & \frac{1}{2} \lambda_{2r_j j} \\ \vdots & \vdots & \vdots & \ddots & \vdots \\ 0 & \frac{1}{2} \lambda_{1r_j j} & \frac{1}{2} \lambda_{2r_j j} & \dots & \lambda_{r_j j} \end{bmatrix} > 0$$

Table 2 The variable responds to the actively managed building with no input

(1)	El Centro (assume PGA = 0.1 g)		Kobe (assume PGA = 0.1 g)	
	Peak (2)	rms (3)	Peak (4)	rms (5)
$x_1(cm)$	3.044	0.843	3.055	1.684
$x_2(cm)$	5.555	1.856	5.466	2.488
$x_3(cm)$	7.265	1.662	6.942	2.749
$\ddot{x}_{1a}(g)$	0.227	0.052	0.223	0.210
$\ddot{x}_{2a}(g)$	0.332	0.057	0.237	0.178
$\ddot{x}_{3a}(g)$	0.361	0.517	0.379	0.213

and $\lambda_M(\bar{Q}_{kj})$ denotes the maximum eigenvalues of \bar{Q}_{kj} . Moreover, $\lambda_m(Q_{ij})$ and $\lambda_m(Q_{ifj})$ denote the minimum eigenvalues of Q_{ij} and Q_{ifj} , respectively.

From the robotic system approximation, a nonlinear plant can be viably approximated and portrayed utilizing the automated model including FLC to accomplish the control object.

For $x \in \xi \subset R^n$, $\bar{w} = [\bar{w}_1, \bar{w}_2, \dots, \bar{w}_n]^T$ satisfies $\bar{u}(S, \bar{w}) = \bar{w}^T R(S)$ approximated with accuracy ε_{max} over the set ξ so that

$$\sup |\bar{u}(S, \bar{w}) - u(S)| \leq \varepsilon_{max} \quad (15)$$

Therefore, the control output could be described as

$$\hat{u}(S, \hat{w}) = \sum_{k=1}^m \hat{w}_k \cdot R_k(S) = \hat{w}^T R(S) \quad (16)$$

and then vector \hat{w} of the genetic algorithm needs to be selected with the initial values at time t by $\tilde{w} = \bar{w} - \hat{w}$, and then

$$\tilde{w}^T R(S) = \bar{u}(S, \bar{w}) - \hat{u}(S, \hat{w}) \quad (17)$$

$$\dot{V} = -\eta \cdot e_m^T Q e_m + 2\eta \cdot S \cdot [g(x) \cdot (\tilde{w}^T R(S) + \varepsilon) - 2\eta \cdot |S| \cdot [g(x) \cdot \tilde{w}^T R(S)] \cdot \text{sat}\left(\frac{S}{\phi}\right)].$$

When $|S| > \Phi$, then $V \leq -\eta \cdot \|e_m\| \cdot [\|e_m\| \cdot Q - 2\|c\| \cdot \|g(x) \cdot \varepsilon\|]$ with $e_m^T P e_m > \phi^2$.

It is true that $V \leq -\eta \cdot \|e_m\| \cdot \sigma$ if $e_m^T P e_m > \phi^2$ and $|S| > \Phi$, and hence $V < 0$. Thus, V will gradually converge to zero as will all the ζ .

According to the Lyapunov stability theory mentioned above, is guaranteed and the tracking error and the modeling error will then both approach zero.

The Evolved Bat Algorithm (EBA) is proposed as an advanced algorithm to automate global bat selection. Unlike other swarm insight calculations, the real purpose of EBA is that there is only one parameter, called the mean, that must be solved for before using the calculation to solve the problem. Selecting different intermediate objects will determine the bubble size for different scenes during the transition. Air was chosen as the medium in this study because it is the first of many different habitats for turtles. The role of the EBA can be divided into two phases: Implementation: Promulgate the dummy model rather than modify it by ordering as needed. Progression: Fake moves.

Generate a random number and check if it is greater than a certain cardiac output. If the result is positive, the pseudo-expert moves with random movements.

$$x_i^f = x_i^{f-1} + D.$$

where x_i^t represents the coordinates of the i -th agent in the t -th iteration and the coordinates of the i -th agent in the last iteration, and D is the distance the agent has moved in this iteration. x_i^{t-1}

$$D = \gamma \cdot \Delta T$$

where γ is the variable associated with the experimentally chosen solution and $\Delta T \in [-1, 1]$ is a random number. $\gamma = 0.17$ The medium of choice was air, so we used it in our experiments.

$$x_i^{tR} = \beta(x_{best} - x_i^t), \beta \in [0, 1]$$

where β is a random number. x_{best} We present the closest optimal structures found so far for all organic compounds. Mark the artificial agent's new coordinates after the random walk process is complete.

5. Numerical simulation and results

To demonstrate the implementation of H_∞ static control in civil buildings, we performed a detailed numerical simulation using a numerical model of a fully seismic 10-story building. The building is rectangular in shape with an area of 4.5×3 meters and a total height of 9 meters (each floor is 3 meters). It is applied on the NCREE vibration table for experimental verification (as shown in Fig. 1). An active gripping system is installed between the first and first floors, which gives the building the strength to withstand earthquakes. The active management structure was tested with LQG management (Huang *et al.* 2021, Jiang *et al.* 2021, Peng *et al.* 2023, Ren *et al.* 2022, Tan *et al.* 2023). A numerical model of the active control structure was successfully proposed, the accuracy of which was confirmed by experimental verification. Therefore, in this article, we use live number modeling as a suitable modeling technique for number simulation. The matrix form of this numerical model can be traced (Shi *et al.* 2023a, b, c, Song *et al.* 2022, Mohammadzadeh *et al.* 2024, Lu *et al.* 2022). This fine model provides relative movement of the ground-related structure for each floor, as well as absolute

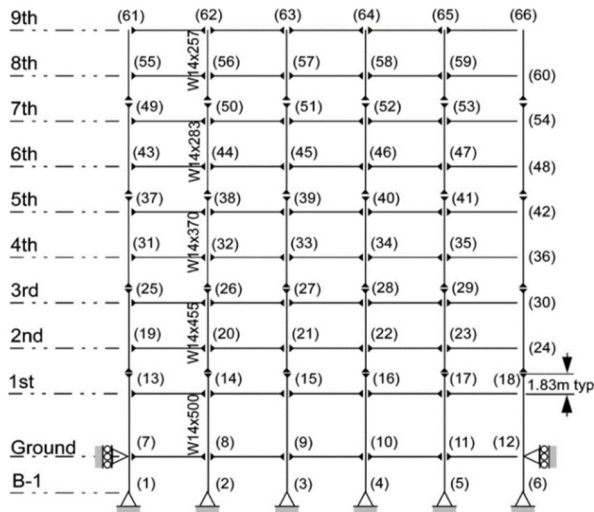


Fig. 7 Values in parentheses indicate percentage reduction compared to free vibration

acceleration for each floor and elevator. The El Centro earthquake in 1940 (100 seconds) and Kobe in 1995 (60 seconds) served as sources of excitement. In this experiment, we used PGA (Peak Ground Acceleration) amplitude obtained at 0.1 g, similar to previous experiments done on the circuit in 2020. It can be easily calculated by multiplying the scale factor.

For comparison, the response of the structure when the earthquake is zero is shown in Table 3. We call this case “out of control”. Because the path and speed parameters are expensive, we chose the absolute acceleration of all three phases as the reaction speed. However, the seven return quantities mentioned above are calculated. In phase, the eight state models used by Luo *et al.* 2023a, b, She *et al.* 2023, Zhou *et al.* 2024 to control LQG are derived from another system model constructed using a minimum of

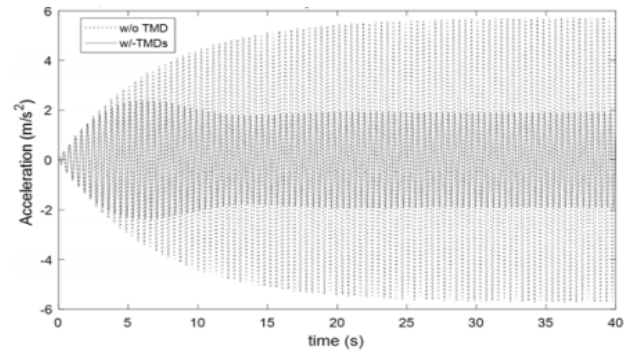


Fig. 8 $\mu = 0.5\%$ Vertical Peak acceleration by proposed controller subjected to external forces

state-of-the-art stability and detection accuracy. It should be noted that it was built. A practical approach. To test the reliability of the controller and compare it with the LQG results, this paper uses the same secondary approach to designing a reliable H_∞ controller. Here, the difference between secondary and actual performance is taken as the uncertainty of the component, and the chosen weight should include all the limitations on the uncertainty of the component.

We then design H_{∞_1} and H_{∞_2} parameters of two controllers that require small control force and large control force are H_{∞_2} chosen H_{∞_1} , and their simulation results are compared with LOG (small control force) and LOG (large control force). $y = [\ddot{x}_{1a}, \ddot{x}_{2a}, \ddot{x}_{3a}]^T$ denotes that unlike proposed controls, the LOG method considers the control function (feedback reduction) as the sole objective of controller design. The weighting functions $W_y = (180 s + 200)/(1.5 s + 750)$ and other parameters γ_{min} used in the LMI calculation for H_{∞_1} each H_∞ controller $\alpha = 0.1589, \beta = 0.1589$ are $W_U = (4.4 s + 200)/(1 s + 440)$: $W_e = (0.02 s + 21,000)/(0.5 s + 10)$. The

Table 3 Control increases the rate of reduction

Error	RCE						
	HN	MN	SN	ZERO	SP	MP	HP
HN	$K_p = \text{dec}$	$K_p = \text{dec}$	$K_p = \text{dec}$	$K_p = \text{dec}$	$K_p = \text{dec}$	$K_p = \text{dec}$	$K_p = \text{dec}$
	$K_i = \text{dec}$	$K_i = \text{med}$	$K_i = \text{inc}$	$K_i = \text{dec}$	$K_i = \text{inc}$	$K_i = \text{med}$	$K_i = \text{dec}$
MN	$K_p = \text{med}$	$K_p = \text{med}$	$K_p = \text{med}$	$K_p = \text{dec}$	$K_p = \text{dec}$	$K_p = \text{dec}$	$K_p = \text{dec}$
	$K_i = \text{med}$	$K_i = \text{med}$	$K_i = \text{inc}$	$K_i = \text{inc}$	$K_i = \text{inc}$	$K_i = \text{inc}$	$K_i = \text{inc}$
SN	$K_p = \text{dec}$	$K_p = \text{dec}$	$K_p = \text{dec}$	$K_p = \text{dec}$	$K_p = \text{dec}$	$K_p = \text{dec}$	$K_p = \text{dec}$
	$K_i = \text{dec}$	$K_i = \text{med}$	$K_i = \text{inc}$	$K_i = \text{med}$	$K_i = \text{inc}$	$K_i = \text{med}$	$K_i = \text{inc}$
ZERO	$K_p = \text{med}$	$K_p = \text{med}$	$K_p = \text{med}$	$K_p = \text{med}$	$K_p = \text{med}$	$K_p = \text{med}$	$K_p = \text{med}$
	$K_i = \text{dec}$	$K_i = \text{med}$	$K_i = \text{inc}$	$K_i = \text{med}$	$K_i = \text{inc}$	$K_i = \text{med}$	$K_i = \text{med}$
SP	$K_p = \text{dec}$	$K_p = \text{inc}$	$K_p = \text{med}$	$K_p = \text{med}$	$K_p = \text{med}$	$K_p = \text{med}$	$K_p = \text{dec}$
	$K_i = \text{dec}$	$K_i = \text{inc}$	$K_i = \text{med}$	$K_i = \text{med}$	$K_i = \text{med}$	$K_i = \text{med}$	$K_i = \text{dec}$
MP	$K_p = \text{med}$	$K_p = \text{inc}$	$K_p = \text{inc}$	$K_p = \text{inc}$	$K_p = \text{inc}$	$K_p = \text{med}$	$K_p = \text{inc}$
	$K_i = \text{dec}$	$K_i = \text{inc}$	$K_i = \text{inc}$	$K_i = \text{dec}$	$K_i = \text{inc}$	$K_i = \text{med}$	$K_i = \text{med}$
HP	$K_p = \text{inc}$	$K_p = \text{inc}$	$K_p = \text{inc}$	$K_p = \text{med}$	$K_p = \text{inc}$	$K_p = \text{inc}$	$K_p = \text{inc}$
	$K_i = \text{dec}$	$K_i = \text{med}$	$K_i = \text{inc}$	$K_i = \text{med}$	$K_i = \text{med}$	$K_i = \text{med}$	$K_i = \text{inc}$

simulated responses for 0.1 g PGA during the two earthquakes using the $H_{\infty 1}$ and $H_{\infty 2}$ controls are listed. Values in parentheses in Fig. 7 indicate percentage reduction compared to ‘no control’ in Table 1 and Fig. 9 with the proposed controller subjected to external forces. From Table 4, we can see that $H_{\infty 1}$ control increases the rate of reduction in RMS response. At 50%, the dominant power is approx. Compared to 1000 kgf, the $H_{\infty 2}$ controller has increased up to 60%, and the operating force is about 1500 kgf. The reduction in peak response was relatively small, and a significant difference in peak reduction was observed between the two different seismic results. For comparison, the simulated responses by the LQG1 and LQG3 controls are also reported in Table 4. The results show that the proposed H_{∞} controller provides better performance comparable to the LQG controller in the control frame structure.

6. Conclusions

Analytical solutions of the crosswind closing behavior from a hypothetical self-contained wind system show that the wind parameters are not only a function of wind size and wind speed, but are largely independent of the weight-to-structure reduction ratio. An LMI-based H_{∞} controller that takes the thickness parameter into account is presented. Robustness criteria include robustness of stability against system uncertainty, and to minimize tracking errors and prevent disturbances and measurement noise. A numerical model of a complete three-story experimental building with an active bracing system is used in simulations to demonstrate that the proposed controller works in a realistic scenario (see Lu *et al.* 2022, Luo *et al.* 2023a, b, Carreras *et al.* 2011 and references therein). Two earthquakes, the El Centro earthquake and the Kobe earthquake, were used in the large-scale simulations. The controller design assumes system uncertainty and for practical reasons the acceleration parameter is used as a feedback quantity. To better demonstrate the potential of the modulation control function in this system, two H_{∞} controllers were designed.

Additionally, we compare the simulation results of the H_{∞} controller with the efficiency of the LQG controller. The simulation results show that (1) the LMI method performs well in computing the H_{∞} controller, and (2) the efficiency of the proposed H_{∞} controller is remarkable, (3) with the ability to reduce interference, tracking error, and noise. Seeing that the performance of the H_{∞} controller as efficient as the LQG controller, the proposed robust H_{∞} control based on LMI is suitable for seismic retrofitting of civil engineering buildings.

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